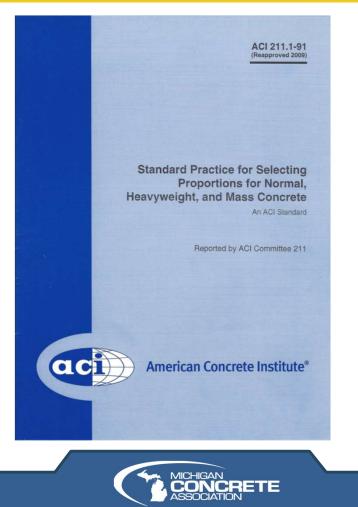


David Hollingsworth – Director of Technical Services/Training

PDF handouts of slides: https://info.miconcrete.org/virtual-learning



Absolute Volume Method

The mix is proportioned to yield 27 ft³ or 1 yd³. Each of the component materials occupies a portion of the overall volume that is determined by dividing the mass (weight) of each material in the mix by the relative density (specific gravity) multiplied by 62.4 lb/ft³, the density of water.

Example: What is the absolute volume (AV) of 564 lbs of Portland cement? $AV = 564 \ \text{lb}/(3.15 \times 62.4 \ \text{lb}/\text{ft}^3) = 2.87 \ \text{ft}^3$



What information do you need to get started? specified compressive strength, f'_c w/c or w/cm ratio (possibly) what is being constructed where in the country is it being constructed contractor requirements

aggregate properties - stone size, fineness modulus - sand, relative density - sand/stone, aggregate absorption, aggregate total moisture, bulk density - stone cementitious properties – relative density of fly ash, slag cement, silica fume



TABLE 4.2.1 — EXPOSURE CATEGORIES AND CLASSES

Category	Severity	Class	Condition				
	Not applicable	F0	Concrete not expo and-thawing cycles				
F	Moderate	F1	Concrete exposed to freezing-and- thawing cycles and occasional exposure to moisture				
Freezing and thawing	Severe	F2	Concrete exposed to freezing-and- thawing cycles and in continuous contact with moisture				
	Very severe	F3	Concrete exposed to freezing-and- thawing and in continuous contact with moisture and exposed to deicing chemicals				
			Water-soluble sulfate (SO ₄) in soil, percent by mass*	Dissolved sulfate (SO ₄) in water, ppm [†]			
s	Not applicable	S0	SO ₄ < 0.10	SO ₄ < 150			
Sulfate	Moderate	S1	0.10 ≤ SO ₄ < 0.20	$150 \le SO_4 < 1500$ Seawater			
	Severe	S2	$0.20 \le \mathrm{SO}_4 \le 2.00$	1500 ≤ SO ₄ ≤ 10,000			
	Very severe	S3	SO ₄ > 2.00	SO ₄ > 10,000			
P Requiring	Not applicable	PO	In contact with water where low permeability is not required				
low permeability	Required	P1	In contact with water where low permeability is required.				
	Not applicable	CO	Concrete dry or protected from moisture				
C Corrosion	Moderate	C1	Concrete exposed to moisture but not to external sources of chloride				
protection of reinforce- ment	Severe	C2	not to external sources of chlorides Concrete exposed to moisture and an external source of chlorides from deicing chemicals, salt, brackish water, seawater, or spray from these sources				



TABLE 4.3.1 — REQUIREMENTS FOR CONCRETE BY EXPOSURE CLASS

Expo- sure Class	Max.	Min. f'_c , psi	Addit	ional minimu	m requiren	nents	
				Air content		Limits on cementi- tious materials	
FO	N/A	2500		N/A			
F1	0.45	4500		Table 4.4.1		N/A	
F2	0.45	4500		Table 4.4.1		N/A	
F3	0.45	4500		Table 4.4.1		Table 4.4.2	
			Cementitie	ous materials	s [†] —types	Calcium	
			 ASTM C150 	ASTM C595	ASTM C1157	chloride admixtur	
S0	N/A	2500	No Type restriction	No Type restriction	No Type restriction	No restriction	
S1	0.50	4000	11‡	IP(MS), IS (<70) (MS)	MS	No restriction	
S2	0.45	4500	V§	IP (HS) IS (<70) (HS)	HS	Not permitted	
S3	0.45	4500	V + pozzolan or slag ^{il}	pozzolan or (<70)		Not permitted	
P0	N/A	2500		Non			
P1	0.50	4000		None			
			chloride content in	ater-soluble ion (CI ⁻) concrete, / weight of ent [#]			
			concrete	Prestressed concrete	Related p	rovisions	
CO	N/A	2500	1.00	0.06	No	ne	
C1	N/A	2500	0.30	0.06			
C2	0.40	5000	0.15	0.06	7.7.6,	18.16	



What if two exposure categories are specified?

Example Concrete needs to meet both exposure class F3 and C2? F3 requires a maximum w/cm of 0.45 and minimum f'_c of 4500 psi while C2 requires a maximum w/cm of 0.40 and minimum f'_c of 5000 psi.

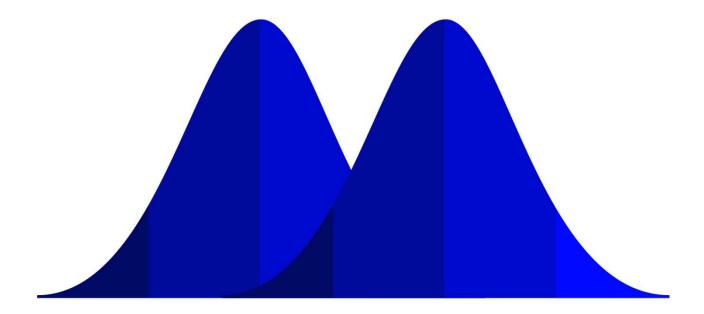
ALWAYS select the most restrictive for developing the mix i.e. C2.



Step 1 Determine the required average compressive strength, f'_{cr} .

Once the licensed design professional has determined the specified compressive strength, f'_{c} , the next step is to determine the required average compressive strength, f'_{cr} . The required average compressive strength is the overdesign that is necessary to MINIMIZE the possibility of the cylinder strength falling below the specified compressive strength.







When field strength test records are not available or you lack sufficient data, the required average compressive strength, f'_{cr} , is determined from Table 4.2.3.1 of ACI 301.

f_c' , psi	f_{cr}' , psi	
Less than 3000	$f_{c}' + 1000$	
3000 to 5000	$f_{c}' + 1200$	
Over 5000	$1.1f_{c}' + 700$	



When field strength test records are available and are not older than 24 months and span no less than 45 calendar days for a class of concrete within 1000 psi of that required for the project, calculate the sample standard deviation, s_s , and determine the required average compressive strength, f'_{cr} , using Table 4.2.3.3(a)1 from ACI 301.

<	f_{cr}' , psi	
f_c' , psi	Use the larger of:	
5000 an loss	$f_{cr}' = f_c' + 1.34 ks_s$	
5000 or less	$f_{cr}' = f_c' + 2.33 k s_s - 500$	
Oncer 5000	$f_{cr}' = f_c' + 1.34 ks_s$	
Over 5000	$f_{cr}' = 0.90 f_c' + 2.33 ks_s$	



If the number of field tests used in calculating the sample standard deviation, s_s , is not 30 or more (but 15 or greater), use Table 4.2.3.3(a)2 to increase the value of the sample standard deviation, s_s .

Total number of tests considered	<i>k</i> -factor for increasing sample standard deviation
15	1.16
20	1.08
25	1.03
30 or more	1.00

Note: Linear interpolation for intermediate number of tests is acceptable.



ACI 211.1-91

- 1.2 The methods provide a first approximation of proportions intended to be checked by trial batches in the laboratory or field and adjusted, as necessary, to produce the desired characteristics of the concrete.
- 2.2 The selection of concrete proportions involves a balance between economy and requirements for placeability, strength, durability, density and appearance. The required characteristics are governed by the use to which the concrete will be put and by conditions expected to be encountered at the time of placement. These characteristics should be listed in the job specifications.



Let's get started....

Type of structurewarehouse floor, 5 inches thick, unreinforcedSpecified strength, f'_c 3500 psiSample std deviation, s_s 644 psi, based on 30 tests



Coarse aggregate properties

Fine aggregate properties

Nominal maximum size
Relative density (dry)
Total moisture
Absorption
Free moisture
Bulk density

1 inch 2.58 3.5% 3.1% 0.4% 97 lb/ft³

Fineness modulus	2.80
Relative density (dry)	2.64
Total moisture	2.5%
Absorption	1.3%
Free Moisture	1.2%



Maximum vs Nominal Maximum Size

maximum size: the smallest sieve opening through which the entire amount of aggregate is **required** to pass.

nominal maximum size: the smallest sieve opening through which the entire amount of aggregate is **permitted** but not required to pass.



Verify choice of nominal maximum aggregate size per ACI 211.1-91.

6.3.2 In no event should the nominal maximum aggregate size exceed:
a. 1/5 the narrowest dimension between sides of forms
b. 1/3 the depth of the slab
c. 3/4 the minimum clear spacing between individual reinforcing bars, bundles of bars or pretensioning strands

1/3 of 5 inches = 1.67 inches maximum > therefore, 1 inch proposed for use is OK



Next, determine required overdesign, f'_{cr} for f'_{c} equal to 3500 psi:

$$\begin{array}{ll} f'_{cr} & = f'_{c} + 1.34 \ \text{ks}_{s} \\ & = 3500 \ \text{psi} + 1.34 \ (1.0) \ 644 \ \text{psi} = 4363 \ \text{psi} \ (\text{k=1.0 for 30 tests}) \\ f'_{cr} & = f'c + 2.33 \ \text{ks}_{s} - 500 \\ & = 3500 \ \text{psi} + 2.33 \ (1.0) \ 644 \ \text{psi} - 500 = 4501 \ \text{psi} \end{array}$$

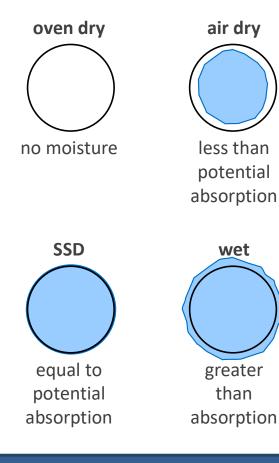
ALWAYS select the larger of the two for proportioning the mix and round to the nearest 500 psi.



To determine mix proportions for first approximation:

- 1. Complete ACI table information
- 2. Calculate dry batch weights
- 3. Calculate dry batch volumes
- 4. Calculate saturated-surface-dry (SSD) weights
- 5. Adjust batch for actual moisture conditions





Aggregates have an internal void or pore structure that at any time may or may not contain water (absorbed water). Depending on the moisture content, the four possible conditions are as follows:

 oven dry - no moisture present (only attainable in lab)
 air dry - dry at surface, capable of absorbing additional water
 saturated-surface dry (SSD) - fully saturated, neither absorbs nor contributes water to the mix

4. **wet** - contains an excess amount of surface moisture



	PREPARI	DATE:	
ACI 211 C/	LCULATIONS FOR CONCRETE PROPO		
STRUCTURE IDENTIFICATION			
SPECIFIED STRENGTH (fc)			
REQUIRED STRENGTH (fcr)			
COARSE AGGREGATE A. CLASSIFICATION B. NOMINAL MAX. SIZE C. RELATIVE DENSITY (dry), RDC D. TOTAL MOISTURE E. ABSORPTION F. FREE MOISTURE G. BULK DENSITY, M	in. B. FIN 	SGREGATE ASSIFICATION ASSIFICATION ASSIFICATION ASSIFICATION ASSIFICATION ASSIFICATION ASSIFICATION ASSIFICATION ASSORPTION ASSORPTI	
ACI TABLES SLUMP (Table 6.3.1) % AIR (Table 6.3.3) WATER (Table 6.3.3)	in. VOL C.A. (Table 6.3.6) % W/C RATIO (Table 6.3.4(a)) b. W/C RATIO (Table 6.3.4(b))	STRENGTH DURABILITY	
1. DRY DESIGN WEIGHTS (per 27 CU.FT.)	2. DRY ABSOLUTE VOLUME	3. SSD DESIGN WEIGHTS (per 27 CU.FT.)	
CEMENT Mix Water () =lb	$\left(\frac{1}{3.15 \times 62.4}\right)$ lb =CU. FT.	CEMENT =lb	
WATER =lb	() lb =CU. FT.	WATER =Ib	
COARSE () × 27 × () =Ib VOL. C.A. M.	$\left(\begin{array}{c} \\ \end{array}\right)$ b = CU. FT. RDC $\left(\begin{array}{c} \\ \end{array}\right)$ X 62.4	COARSE AGGREGATE =lb (Dry Wt.) x (1 + <u>(ABS)</u>) 100	
AIR (%) x 27 CU.FT. = entrapped or (%) total)	=CU. FT.	FINE AGGREGATE = lb (Dry Wt.) x (1 + (ABS)) 100	
FINE () X () X 62.4 =lb ABS. VOL, RDF	SUB TOTAL =CU. FT.	TOTAL WEIGHT =Ib	
1	(ABS. VOL. FINE) = CU. FT.	DENSITY =Ib/ft (Total Wt./27.0 ft ³)	r -
MOISTURE CORRECTIONS: (DRY WEIGHT) x (%	FREE MOISTURE) + SSD WEIGHT = WET WEI	GHT (Batch Wts.)	
EMENT		BATCH WTS.	
COARSE AGGREGATE (DRY) x	(%FREE) = (Ib WATER) +	(SSD) = Ib (WET)	
INE AGGREGATE (DRY) x	(%FREE) = (Ib WATER)	(SSD) = Ib (WET)	
VATER (SSD WATER)	- (Ib WATER)	(000) = ID (WET) = Gal	



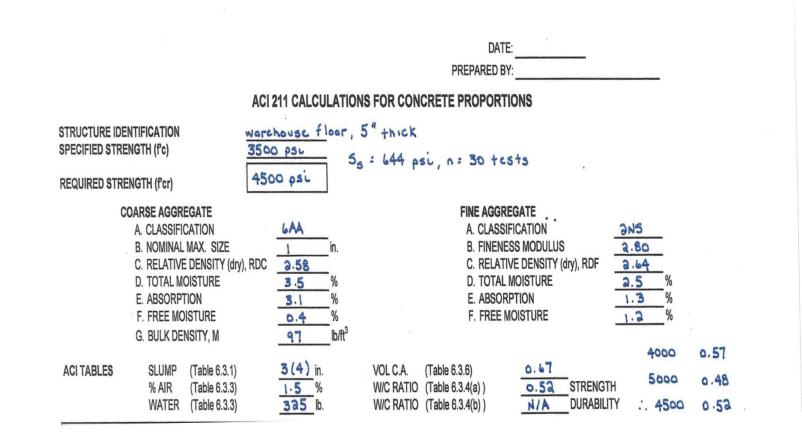




Table 6.3.1 — Recommended slumps for various types of construction*

é	Slump, in.			
Types of construction	Maximum'	Minimum		
Reinforced foundation walls and footings	3	the second		
Plain footings, caissons, and substructure walls	3	1		
Beams and reinforced walls	4	1		
Building columns	4	1		
Pavements and slabs	3	1		
Mass concrete	2	1		

*Slump may be increased when chemical admixtures are used, provided that the admixture-treated concrete has the same or lower water-cement or watercementitious material ratio and does not exhibit segregation potential or excessive bleeding.

'May be increased 1 in. for methods of consolidation other than vibration.



Table 6.3.3 — Approximate mixing water and air content requirements for different slumps and nominal maximum sizes of aggregates

Water, lb/yd3 of concr	ete for i	ndicated	l nomin	al max	imum size	s of agg	regate	
Slump, in.	3/s in.*	1/2 in.*	3/4 in.*	1 in.*	1-1/2 in.*	2 in.*.'	3 in.'"	6 in.'.
	Non-a	air-entra	ined co	ncrete				
1 to 2 3 to 4	350 385	335 365	315 340	300 325	275 300	260 285	220 245	190 210
6 to 7	410	385	360	340	315	300	270	_
More than 7*	-	-	-	_	_	-	-	-
Approximate amount of entrapped air in non-air-entrained concrete, percent	3	2.5	2	1.5	1	0.5	0.3	0.2
	Air	-entrain	ed conc	rete				
1 to 2	305	295	280	270	250	240	205	180
3 to 4	340	325	305	295	275	265	225	200
6 to 7	365	345	325	310	290	280	260	-
More than 7* Recommended averages' total air content, percent for level of exposure:	-	-	-	_	-	-	-	_
Mild exposure	4.5	4.0	3.5	3.0	2.5	2.0	1.5**."	1.0**.**
Moderate exposure	6.0	5.5	5.0	4.5	4.5	4.0	3.5**."	3.0**."
Severe exposure ¹¹	7.5	7.0	6.0	6.0	5.5	5.0	4.5**."	4.0**."



Table 6.3.6 — Volume of coarse aggregate per unit of volume of concrete

Nominal maximum size	agg	regate [*] per e for differe	dry-rodded unit volument fineness ggregate'	e of
of aggregate, in.	2.40	2.60	2.80	3.00
¥8	0.50	0.48	0.46	0.44
1/2	0.59	0.57	0.55	0.53
3/4	0.66	0.64	0.62	0.60
1	0.71	0.69	0.67	0.65
11/2	0.75	0.73	0.71	0.69
2	0.78	0.76	0.74	0.72
3	0.82	0.80	0.78	0.76
6	0.87	0.85	0.83	0.81

*Volumes are based on aggregates in oven-dry-rodded condition as described in ASTM C 29.

These volumes are selected from empirical relationships to produce concrete with a degree of workability suitable for usual reinforced construction. For less workable concrete, such as required for concrete pavement construction, they may be increased about 10 percent. For more workable concrete see Section 6.3.6.1.

'See ASTM C 136 for calculation of fineness modulus.



Table 6.3.4(a) — Relationship between watercement or water-cementitious materials ratio and compressive strength of concrete

	Water-cement ratio, by weight				
Compressive strength at 28 days, psi*	Non-air-entrained concrete	Air-entrained concrete			
6000	0.41	-			
5000	0.48	0.40			
4000	0.57	0.48			
3000	0.68	0.59			
2000	0.82	0.74			



Table 6.3.4(b) — Maximum permissible watercement or water-cementitious materials ratios for concrete in severe exposures*

Type of structure	Structure wet continuously or frequently and exposed to freezing and thawing'	Structure exposed to sea water or sulfates
Thin sections (railings, curbs, sills, ledges, ornamental work)		
and sections with less than 1 in. cover over steel	0.45	0.40*
All other structures	0.50	0.451

*Based on report of ACI Committee 201. Cementitious materials other than cement should conform to ASTM C 618 and C 989.

[†]Concrete should also be air-entrained.

³If sulfate resisting cement (Type II or Type V of ASTM C 150) is used, permissible water-cement or water-cementitious materials ratio may be increased by 0.05.



Note: When looking at the w/c (w/cm) ratio for both strength (Table 6.3.4 (a)) AND durability (Table 6.3.4 (b)) – for example an exterior slab in a cold weather environment - the w/c (w/cm) that is selected for mix proportioning will always be the one that is most restrictive.

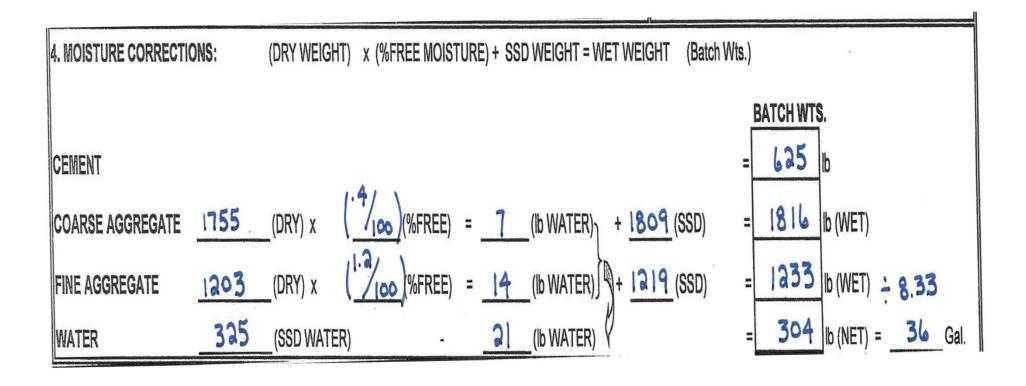


1. DRY DESIGN WEIGHTS (per 2	7 CU.F	т.) 🖂		2. DRY ABSOL	
CEMENT Mix Water (<u>325</u>) W/C Ratio (<u>0.52</u>)	= .	625	lb	(635) lb 3.15 X 62.4	= <u>3.18</u> CU.FT.
WATER	=	325	lb	(325) lb 1.00 X 62.4	= <u>5.21</u> CU.FT.
COARSE (0.67) x 27 x (97) VOL. C.A. M.	=	1755	lb	(<u>1755</u>) lb (2.58) X 62.4 RDC	= <u>10.90</u> CU. FT.
AIR (<u>1.5</u> %) x 27 CU. (entrapped or (<u>100</u> %)	FT.	=			= <u>0.41</u> CU. FT.
total)					
FINE (7.30) x (2.44) x 62.4	=	1203		SUB TOTAL	= 19.70 CU. FT.
ABS. VOL. RDF			-	27.0 - SUB TOTAL (ABS. VOL. FINE)	= <u>7.30</u> CU. FT.



3. SSD DESIGN WEIGHTS (per 27 CU.FT.)				
CEMENT	=	625 lb		
WATER	=	<u>325</u> lb		
COARSE AGGREGATE (Dry Wt.) x (1 + <u>(ABS)</u>) 100	=,	1809 lb		
$1755 \times (1 + 3.1/100)$ FINE AGGREGATE (Dry Wt.) x (1 + (ABS))) =	1219 b		
1203 × (1+ 1.3/100)	×		
TOTAL WEIGHT	=	3978 lb		
DENSITY (Total Wt./27.0 ft ³)	=	147.3 lb/ft ³		







	DATE
PREPAI	DATE:
ACI 211 CALCULATIONS FOR CONCRETE PROF	PORTIONS
STRUCTURE IDENTIFICATION Warehouse floor, 5" +hick SPECIFIED STRENGTH (Pc) 3500 psc	
REQUIRED STRENGTH (for) 4500 psi	+ + + + 5 + 5
A. CLASSIFICATION CAA A. CL B. NOMINAL MAX. SIZE C. RELATIVE DENSITY (dry), RDC D. TOTAL MOISTURE 3.5 E. ABSORPTION 3.1 % E. A	AGGREGATE LASSIFICATION 2N5 LASSIFICATION 3.60 ELATIVE DENSITY (dry), RDF 07AL MOISTURE 3.5 BSORPTION 1.3 % REE MOISTURE 1.2 %
ACI TABLES SLUMP (Table 6.3.1) 3 (4) in. VOL C.A. (Table 6.3.6) % AIR (Table 6.3.3) 1.5 % W/C RATIO (Table 6.3.4(a)) WATER (Table 6.3.3) 335 b. W/C RATIO (Table 6.3.4(b))	0.67 0.52 STRENGTH 5000 0.48 N/A DURABILITY ∴ 4500 0.53
1. DRY DESIGN WEIGHTS (per 27 CU.FT.) 2. DRY ABSOLUTE VOLUME	3. SSD DESIGN WEIGHTS (per 27 CU.FT.)
CEMENT MX Water $(\frac{325}{0.52})$ = 225 b $(\frac{235}{3.15 \times 62.4})$ b = 3.18 CU. FT	T. CEMENT = _635_1b
WATER = <u>325</u> b (325) b = <u>5.21</u> CU. FT 1.00 X 62.4	т. WATER = <u>за5</u> ь
COARSE (0. 67) × 27 × (97) = <u>1755</u> b (<u>1755</u>) b = <u>10.90</u> CU. FT VOL. C.A. M. (2.58) X 62.4	100
AIR (entrapped or (100 %) (1.5 %) x 27 CU,FT. x 27 CU,FT. = = <td>(Dry Wt.) x (1 + (ABS))</td>	(Dry Wt.) x (1 + (ABS))
	1203 × (1+ 1.3/100)
FINE (7.30)x(2.44) x 62.4 = 1203 b SUB TOTAL = 19.70 CU. FT ABS. VOL. RDF 27.0 - SUB TOTAL	T. TOTAL WEIGHT = <u>3978</u> b DENSITY = <u>147.3</u> b/ft ³
(ABS. VOL. FINE) = 7.30 CU. FT	T. (Total Wt./27.0 ft ³)
4. MOISTURE CORRECTIONS: (DRY WEIGHT) × (%FREE MOISTURE) + SSD WEIGHT = WET WE	EIGHT (Batch Wts.)
CEMENT	BATCH WTS. =b
COARSE AGGREGATE _1755 (DRY) x $\left(\frac{17}{100}\right)(\% \text{FREE}) = 7$ (b WATER) + 18	809 (SSD) = 1816 (WET)
FINE AGGREGATE 1203 (DRY) x (1.200)%FREE) = 14 (16 WATER)	
WATER 325 (SSD WATER) - 21 (Ib WATER)	= 304 lb (NET) = 36 Gal.



	Design Weights, Ibs	Batch Weights, Ibs
Cement	625	625
Water	325	304
Sand	1219 (SSD)	1233
Stone	(D22) (2011)	1816
	Slump 3-4 inches	

Air 1.5% Density 147.3 lb/ft³



Are we finished? Not quite.

The mix must ALWAYS be checked by trial batches in the laboratory or field and adjusted, as necessary, to produce the desired characteristics. Specifically, we will be evaluating slump (3-4 inches), air (1.5%), density (147.3 lb/ft³), compressive strength (4500 psi), placeability and appearance (how does the mix look),





dhollingsworth@miconcrete.net 734.216.1221

ALSO, PLEASE SEND <u>SUGGESTIONS</u> FOR ADDITIONAL CONCRETE WEBINAR TOPICS!

For the current webinar schedule: https://info.miconcrete.org/virtual-learning

